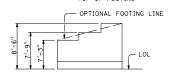
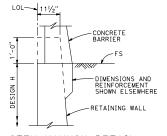


USE Reinf FOR H= 4'-0, 6'-0, 8'-0, FOR JOINTS REQUIRED, SEE $\frac{(B0-3)}{3-3}$ TOP OF WALL TOE OF SLOPE-

TOP OF FOOTING



TYPICAL LAYOUT EXAMPLE



STEM HAUNCH DETAIL

SYMBOLS:

Ser - service limit state I Str - strength limit state I

Ext I - extreme event limit state I

B' - effective footing width (ft)

 q_0' - net bearing stress (ksf), OG assumed to be FG at toe

qo - gross uniform bearing stress (ksf)

TABLE OF REINFORCING STEEL, DIMENSIONS AND DATA				
4'	6′	8′	10'	12'
7'-3"	7'-9"	8'-6"	9'-6"	10'-6"
NONE	NONE	100:2	100:3	100:4
#7 @ 12	#7 @ 10	#7 @ 12	#7 @ 12	#7 @ 10
#7 @ 12	#7 @ 10	#8 @ 12	#9 @ 12	#10 @ 10
6.2, 1.4	6.1, 1.8	6.4, 2.1	7.0, 2.5	7.7, 2.8
6.2, 2.4	6.1, 2.9	5.3, 3.0	6.0, 3.5	6.6, 4.0
4.4, 1.5	4.1, 2.2	4.0, 3.1	4.1, 3.9	4.2, 4.8
2.5, 2.7	3.1, 3.0	3.8, 3.2	4.9, 3.3	5.8, 3.5
	4′ 7′-3" NONE #7 @ 12 #7 @ 12 6.2, 1.4 6.2, 2.4 4.4, 1.5	4' 6' 7'-3" 7'-9" NONE NONE #7 @ 12 #7 @ 10 #7 @ 12 #7 @ 10 6.2, 1.4 6.1, 1.8 6.2, 2.4 6.1, 2.9 4.4, 1.5 4.1, 2.2	4' 6' 8' 7'-3" 7'-9" 8'-6" NONE NONE 100 : 2 #7 @ 12 #7 @ 10 #7 @ 12 #7 @ 12 #7 @ 10 #8 @ 12 6.2, 1.4 6.1, 1.8 6.4, 2.1 6.2, 2.4 6.1, 2.9 5.3, 3.0 4.4, 1.5 4.1, 2.2 4.0, 3.1	4' 6' 8' 10' 7'-3" 7'-9" 8'-6" 9'-6" NONE NONE 100 : 2 100 : 3 #7 @ 12 #7 @ 10 #7 @ 12 #7 @ 12 #7 @ 12 #7 @ 10 #8 @ 12 #9 @ 12 6.2, 1.4 6.1, 1.8 6.4, 2.1 7.0, 2.5 6.2, 2.4 6.1, 2.9 5.3, 3.0 6.0, 3.5 4.4, 1.5 4.1, 2.2 4.0, 3.1 4.1, 3.9

DESIGN CONDITIONS:

Design H may be exceeded by 6" before going to the next size. Special footing design is required where foundation material is incapable of supporting bearing stress listed in the table.

yang Deny

May 31, 2018

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PLANS APPROVAL DATE

POST MILES TOTAL PROJECT

Gary Wang

C58298

Exp. 6-30-18

CIVIL

DESIGN NOTES:

DESIGN: AASHTO LRFD Bridge Design Specifications, 4th Edition with California Amendments LS: Varied surcharge on level ground surface

Stem Architectural Treatment of thickness up to 6" of concrete (75 psf) considered DC:

54 kip transverse force applied at He = 32", distributed over 10 feet at the top of wall and 1: 1 distribution down and outward. Distribution below footing taken no less than 40'. CT:

SEISMIC:

 $\phi = 34^{\circ}$ $\gamma = 120 \text{ pcf}$ SOIL:

REINFORCED CONCRETE:

Q = 1.00DC+1.00EV+1.00EH+1.00CT Where:

Force Effects
1.25 or 0.90, Whichever Controls Design
1.35 or 1.00, Whichever Controls Design
1.50 or 0.90, Whichever Controls Design
Dead Load of Structure Components
Horizontal Earth Fill Pressure
Vertical Earth Pressure from Earth Fill Weight
Live Load Surcharge
Selsmic Earth Pressure
Soli and Structural and Nonstructural Components Inertia
Venicular Collision Force β: η: DC:

ĒĤ:

LS: EQE: EQD: CT:

NOTES:

1. At @ bars:

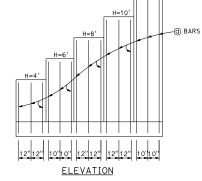
H ≤ 6', no splices are allowed within 1'-8" above the top of footing. H > 6', no splices are allowed within H/4 above the top of footing.

2. Provide #6 @ 8" @ bars in addition to tabulated @ bars over a distance of 8'-0" measured from all expansion joints, begin wall and end wall locations.

RETAINING WALL TYPE 5 (CASE 1)

NO SCALE

B3-4A



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Ext II - extreme event limit state II